# HBS PLATE EVO

PAN HEAD SCREW

#### HBS P EVO

Designed for outdoor steel-to-timber joints: the thickness of the shoulder screw is increased for completely safe, reliable fastening plates to the timber. The small sizes (5,0 and 6,0 mm) are also ideal for timber-to-timber joints.

#### C4 EVO COATING

20 µm multilayer coating with a surface treatment of epoxy resin and aluminium flakes. No rust after 1440 hours of salt spray exposure, as per ISO 9227. Can be used in service class 3 outdoor applications and under class C4 atmospheric corrosion conditions.

#### AGGRESSIVE WOODS

Ideal for applications with woods containing tannin or treated with impregnating agents or other chemical processes.



## CHARACTERISTICS

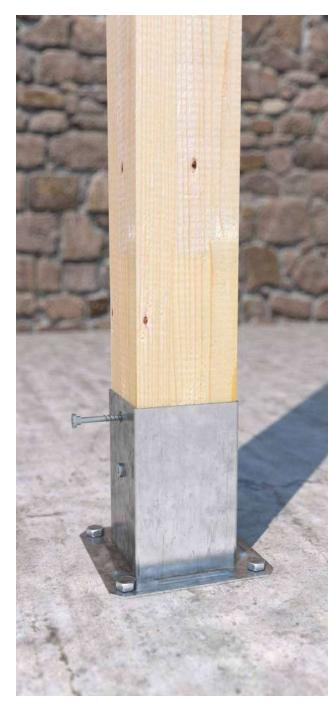
FOCUS	corrosiveness class C4
HEAD	shoulder for plate
DIAMETER	from 5,0 to 10,0 mm
LENGTH	from 40 to 180 mm











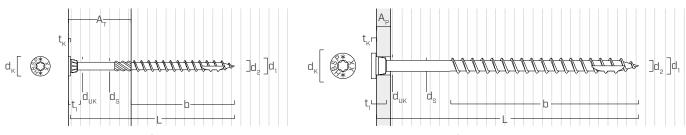
# MATERIAL

Carbon steel, with a 20  $\mu m$  coating, highly resistant to corrosion.

## FIELDS OF USE

- timber based panels
- solid timber and glulam
- CLT, LVL
- high density woods
- aggressive woods (containing tannin)
- chemically treated woods
- Service classes 1, 2 and 3.

# GEOMETRY AND MECHANICAL CHARACTERISTICS



HBS P EVO - 5,0 | 6,0 mm

HBS P EVO - 8,0 | 10,0 mm

Nominal diameter	d1	[mm]	5	6	8	10
Head diameter	d <sub>K</sub>	[mm]	9,65	12,00	14,50	18,25
Tip diameter	d <sub>2</sub>	[mm]	3,40	3,95	5,40	6,40
Shank diameter	ds	[mm]	3,65	4,30	5,80	7,00
Head thickness	t <sub>1</sub>	[mm]	5,50	6,50	8,00	10,00
Washer thickness	t <sub>K</sub>	[mm]	1,00	1,50	3,40	4,35
Underhead diameter	d <sub>UK</sub>	[mm]	6,0	8,0	10,00	12,00
Pre-drilling hole diameter <sup>(1)</sup>	d <sub>V</sub>	[mm]	3,0	4,0	5,0	6,0
Characteristic yield moment	M <sub>y,k</sub>	[Nm]	5,4	9,5	20,1	35,8
Characteristic withdrawal-resistance parameter <sup>(2)</sup>	f <sub>ax,k</sub>	[N/mm <sup>2</sup> ]	11,7	11,7	11,7	11,7
Associated density	ρ <sub>a</sub>	[kg/m <sup>3</sup> ]	350	350	350	350
Characteristic head-pull-through parameter <sup>(2)</sup>	f <sub>head,k</sub>	[N/mm <sup>2</sup> ]	10,5	10,5	10,5	10,5
Associated density	ρ <sub>a</sub>	[kg/m <sup>3</sup> ]	350	350	350	350
Characteristic tensile strength	f <sub>tens,k</sub>	[kN]	7,9	11,3	20,1	31,4

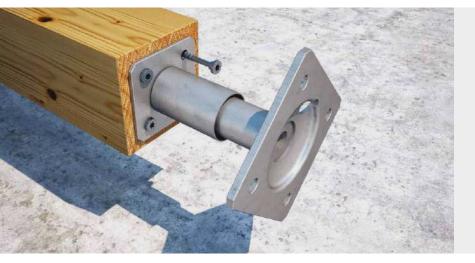
<sup>(1)</sup> Pre-drilling valid for softwood.

<sup>(2)</sup> Valid for softwood - maximum density 440 kg/m<sup>3</sup>.

For applications with different materials or with high density please see ETA-11/0030.

# CODES AND DIMENSIONS

d	CODE	L	b	A <sub>T</sub>	A <sub>P</sub>	pcs		d	CODE	I	L	b	A <sub>P</sub>	pcs
[mm] [in]		[mm] [in]	[mm]	[mm]	[mm]		[r	mm] [in]		[mm]	[in]	[mm]	[mm]	
	HBSPEVO550	50 115/16	30	20	1.0 ÷ 10.0	200		8	HBSPEVO8120	120	4 3/4	95	1.0 ÷ 15.0	100
<b>5</b> 0.20	HBSPEVO560	60 2 3/8	35	25	1.0 ÷ 10.0	200		0.32	HBSPEVO8140	140	5 1/2	110	1,0 ÷ 20,0	100
TX 25	HBSPEVO570	70 2 3/4	40	30	$1.0 \div 10.0$	100		TX 40	HBSPEVO8160	160	6 1/4	130	1,0 ÷ 20,0	100
	HBSPEVO580	80 31/8	50	30	1.0 ÷ 10.0	100			HBSPEVO1060	60	2 3/8	52	1.0 ÷ 15.0	50
6	HBSPEVO680	80 31/8	50	30	1.0 ÷ 10.0	100			HBSPEVO1080	80	3 1/8	60	1.0 ÷ 15.0	50
0.24 <b>TX 30</b>	HBSPEVO690	90 31/2	55	35	1.0 ÷ 10.0	100		10	HBSPEVO10100	100	4	75	1.0 ÷ 15.0	50
17.30	HBSPEVO840	40 19/16	32	-	1.0 ÷ 15.0	100		0.40	HBSPEVO10120	120	4 3/4	95	1.0 ÷ 15.0	50
8	HBSPEVO860	60 2 3/8	52	_	$1.0 \div 15.0$			TX 40	HBSPEVO10140	140	5 1/2	110	1,0 ÷ 20,0	50
0.32	HBSPEVO880	80 31/8	55	-	$1.0 \div 15.0$				HBSPEVO10160	160	6 1/4	130	1,0 ÷ 20,0	50
TX 40	HBSPEVO8100	100 4	75	-	$1.0 \div 15.0$ $1.0 \div 15.0$				HBSPEVO10180	180	7 1/8	150	1,0 ÷ 20,0	50



# TYP R

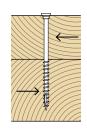
Ideal for fastening standard Rothoblaas plates in outdoor environments.

The 5 mm diameter version is ideal for fastening patio deck planks.

# MINIMUM DISTANCES FOR SHEAR LOADS



Load-to-grain angle  $\alpha = 0^{\circ}$ 



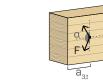
Load-to-grain angle  $\alpha = 90^{\circ}$ 

		SCREWS	INSERTED	WITH PRE-	DRILLING H	SCREWS INSERTED WITH PRE-DRILLING HOLE						
d1	[mm]		5	6	8	10		5	6	8	10	
a <sub>1</sub>	[mm]	5∙d	25	30	40	50	4·d	20	24	32	40	
a <sub>2</sub>	[mm]	3∙d	15	18	24	30	4·d	20	24	32	40	
a <sub>3,t</sub>	[mm]	12·d	60	72	96	120	7∙d	35	42	56	70	
a <sub>3,c</sub>	[mm]	7∙d	35	42	56	70	7∙d	35	42	56	70	
a <sub>4,t</sub>	[mm]	3·d	15	18	24	30	7∙d	35	42	56	70	
a <sub>4,c</sub>	[mm]	3·d	15	18	24	30	3∙d	15	18	24	30	

		SCREWS IN	SCREWS INSERTED WITHOUT PRE-DRILLING HOLE								
d1	[mm]		5	6	8	10		5	6	8	10
a <sub>1</sub>	[mm]	12·d	60	72	96	120	5∙d	25	30	40	50
a <sub>2</sub>	[mm]	5·d	25	30	40	50	5∙d	25	30	40	50
a <sub>3,t</sub>	[mm]	15·d	75	90	120	150	10·d	50	60	80	100
a <sub>3,c</sub>	[mm]	10·d	50	60	80	100	10·d	50	60	80	100
a <sub>4,t</sub>	[mm]	5·d	25	30	40	50	10·d	50	60	80	100
a <sub>4,c</sub>	[mm]	5∙d	25	30	40	50	5∙d	25	30	40	50

d = nominal screw diameter

stressed end -90° < α < 90°



]a,

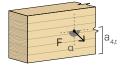
\_la,

a, a

unloaded end 90° < α < 270°



stressed edge 0° < α < 180°



unload edge 180° < α < 360°



#### NOTES:

- Minimum distances are in accordance with EN 1995:2014 as per ETA-11/0030 considering a timber characteristic density of  $\rho_k \le 420~\text{kg/m}^3.$
- In the case of joints with elements in Douglas fir, the minimum spacing and distances parallel to the grain must be multiplied by a coefficient of 1.5.
- The minimum spacing for all steel-to-timber connections  $(a_{1},\,a_{2})$  can be multiplied by a coefficient of 0,7.
- The minimum spacing for all panel-to-timber connections(a<sub>1</sub>, a<sub>2</sub>) can be multiplied by a coefficient of 0,85.

#### STATIC VALUES

CHARACTERISTIC VALUES
EN 1995:2014

						TENSION								
geometry				timber-to-timber	panel-to-timber <sup>(1)</sup>			thin steel-timber plate <sup>(2)</sup> plate <sup>(3)</sup>				thread withdrawal <sup>(4)</sup>	head pull-through <sup>(5)</sup>	
d1	L	b	А	R <sub>V,k</sub>	R	/,k	R	/,k	R <sub>V,k</sub>		R	/,k	R <sub>ax,k</sub>	R <sub>head,k</sub>
[mm]	[mm]	[mm]	[mm]	[kN]	[k	N]	[kl	N]	[kN]		[kN]		[kN]	[kN]
	50	30	20	1,29		1,05		1,12		1,74		2,25	2,03	1,13
5	60	35	25	1,43	S <sub>PAN</sub> = 9 mm	1,05	S <sub>PAN</sub> = 12 mm	1,12	S <sub>PLATE</sub> = 2,5 mm	1,82	S <sub>PLATE</sub> = 5,0 mm	2,33	2,37	1,13
	70	40	30	1,51	9 P	1,05		1,12	S <sub>PL</sub> 2,5	1,91	S <sub>PL</sub> 5,0	2,42	2,71	1,13
	80	50	30	1,51		1,05		1,12		2,08		2,59	3,38	1,13
	80	50	30	2,02	= 2	1,51	= 2	1,58	Ju =	2,76	ш Щ	3,48	4,06	1,75
6	90	55	35	2,18	S <sub>PAN</sub> = 12 mm	1,51	S <sub>PAN</sub> = 15 mm	1,58	S <sub>PLATE</sub> = 3,0 mm	2,86	S <sub>PLATE</sub> = 6,0 mm	3,58	4,47	1,75
	40	32	8	1,18		-		-		2,13		3,66	3,47	2,55
	60	52	8	1,18	F	-	Ę	-	E	3,31	E	5,12	5,63	2,55
	80	55	25	2,67	= 15 mm	2,32	S <sub>PAN</sub> = 18 mm	2,38	= 4,0 mm	4,29	8,0 mm	5,45	5,96	2,55
8	100	75	25	2,67	= 15	2,32	= 18	2,38	= 4	4,83	11	5,99	8,12	2,55
	120	95	25	2,67	SPAN :	2,32	PAN	2,38	S <sub>PLATE</sub>	5,37	S <sub>PLATE</sub>	6,53	10,29	2,55
	140	110	30	2,83	0	2,32	0	2,38	S	5,60	S	6,94	11,91	2,55
	160	130	30	2,83		2,32		2,38		5,60		7,48	14,08	2,55
	60	52	8	1,38		-		-		3,80		6,31	7,04	4,05
	80	60	20	3,45	E	2,55	E	3,12	,0 mm	5,18	E E	7,74	8,12	4,05
	100	75	25	3,77	= 15 mm	2,55	= 18 mm	3,12	0 n	6,56	= 10,0 mm	8,26	10,15	4,05
10	120	95	25	3,77		2,55	= 1{	3,12	= 5,	7,26	= 1(	8,93	12,86	4,05
	140	110	30	3,91	SPAN	2,55	SPAN	3,12	S <sub>PLATE</sub>	7,77	SPLATE -	9,44	14,89	4,05
	160	130	30	3,91	0	2,55	0	3,12	SP	8,09	Spl	10,12	17,60	4,05
	180	150	30	3,91		2,55		3,12	L	8,09		10,80	20,31	4,05

#### NOTES:

- $^{(1)}$  The characteristic shear resistances are calculated considering an OSB3 or OSB4 panel, as per EN 300, or a particle board panel, as per EN 312, with thickness S<sub>PAN</sub>.
- <sup>(2)</sup> The shear resistance characteristics are calculated considering the case of a thin plate (S<sub>PLATE</sub>  $\leq$  0,5 d<sub>1</sub>).
- $^{\rm (3)}$  The shear resistance characteristics are calculated considering the case of a thick plate ( $S_{PLATE} \ge d_1$ ).
- <sup>(4)</sup> The axial thread withdrawal resistance was calculated considering a 90° angle between the grain and the connector and for a fixing length of b.
- <sup>(5)</sup> The axial resistance to head pull-through was calculated using timber elements.

In the case of steel-to-timber connections, generally the steel tensile strength is binding with respect to head separation or pull-through.

#### **GENERAL PRINCIPLES:**

- · Characteristic values comply with the EN 1995:2014 standard in accordance with ETA-11/0030.
- Design values can be obtained from characteristic values as follows:

$$R_d = \frac{R_k \cdot K_{mod}}{\gamma_M}$$

- The coefficients  $\gamma_M$  and  $k_{mod}$  should be taken according to the current regulations used for the calculation.
- For the mechanical resistance values and the geometry of the screws, reference was made to ETA-11/0030.
- For the calculation process a timber characteristic density  $\rho_k$  = 420 kg/m<sup>3</sup> has been considered.
- Values were calculated considering the threaded part as being completely inserted into the wood.
- Sizing and verification of the timber elements, panels and steel plates must be done separately.
- The characteristic shear resistances are calculated for screws inserted without pre-drilling hole. In the case of screws inserted with pre-drilling hole, greater resistance values can be obtained.
- For different calculation configurations, the MyProject software is available (www.rothoblaas.com).